

# NEES@UCLA: Advanced Dynamic Field Testing of Civil Structures



# Who are we?

### o Principal Investigators are:

- John Wallace
- Jon Stewart
- Robert Nigbor
- Professional Staff:
  - Steve Keowen Mechanical engineer
  - Alberto Salamanca Instrumentation
  - Steve Kang IT
  - Sophia Poulos Instrumentation
  - Erica Eskes Administration

# **Equipment Portfolio**

- Vibration sources (shakers)
- Data acquisition & sensors
- **O CPT Truck & RSA**
- High performance mobile network





# **Vibration Sources**

### 0 Eccentric mass shakers

- MK14A (1x)
  - omni-directional, 0 to 4.2 Hz & 15 kips
- MK15 (2x)
  - uni-directional, 0 to 25 Hz & 100 kips
  - Synchronized 200 kips
  - AFB

- Uni-directional, 0 20 Hz & 10 kips
- Fits in a pickup truck and elevator

### Linear inertial shaker

- Digital controllers
- 15 kips, ± 15 inches & 78 in/s





# Data Acquisition and Sensors

### o Kinemetrics

- Q330 data loggers (120 channels total)
- Episensor accelerometers
- 24-bit, large dynamic bandwidth ~135 dB
- GPS time synchronization
- Wireless telemetry using 802.11a/b
- o National Instruments
  - SCXI/PXI combo chassis (96 channels)
  - 16 bit resolution
  - GPS time synchronization
  - Strain gauges, displacement transducers



# High Performance Mobile Network

### Mobile Command Center

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- T1 satellite uplink (1.54 Mbps)
  - UNIX workstations
- o Networking Equipment
  - Wireless Field-LAN
  - Campus-LAN
  - Satellite transmission system





# **UCLA Four Seasons Project**

- Forced-Vibration Testing
  - Sherman Oaks, California
  - 4-story RC Building (1977)
- Damaged (yellow tag) in Northridge earthquake
  - Empty, to be demolished
- o Complete System Test
  - Shakers/Sensors & DAQ (200 sensor channels)
  - Mobile command center
  - Satellite, Tele-presence





# Building Description

- Perimeter Moment Resisting Frame
  - Beam : 24"x30"
  - Column : 24"x24"
- o Gravity Load :
  - Post-tensioned slab with drop panels (8 1/2")
  - interior columns
- Bell caisson foundation



**Typical Floor Plan** 

## Four Seasons Building Vibration Tests



# Building Shaking Example: Four Seasons Building



UCLA's large shakers:

100,000 lbs dynamic force each



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# **Earthquake-Level Shaking**



### Caltrans Field Testing at the Caltrans-UCLA Test Site (105 & 405)



# Caltrans 9-Pile Group





# Basic Nonlinear Stiffness Curve (up to 3/4")



# LAX Theme Building Assessment



Los Angeles World Airports LAX - Theme Building Restoration





# Ministry Structural & ARTHQUAKE ENGINEERING VCA Engineers Inc. CSA Constructors





### **Events and Chronology**





#### NEES@UCLA Tests NEES@UCLA Tests

# LAX Theme Building EMA

- o EMA = "Experimental Modal Analysis"
- The purpose of EMA is to measure the dynamic properties of a real structure for comparison with and validation of computer models of the structure
- EMA is common in mechanical & aerospace engineering, not so common in civil engineering



### Measurements

- UCLA's small shaker, with 10,000 lb maximum force, installed on east side of observation deck. Force set to (100 x f<sup>2</sup>) lbs.
- **o** 51 channels of accelerometers installed at 18 locations
- Very high resolution digital recording to measure ambient through earthquake levels (micro-g to 2g)





#16 at ground level, vertical







# Data Recording

- o Thursday Oct. 18: Installation
- Friday Oct. 19: E-W (X) shaking
- Friday–Sunday: Ambient Vibration, Santa Ana winds on Saturday Oct. 20 evening to 20 mph
- Monday Oct. 22: N-S and E-W shaking
- Monday–Friday: Ambient vibration, continuous

### Sample Data:

Location 14, observation deck, vertical, 1-hour, ambient & shaking

Peak~0.01g



Spectrogram, Frequency & Amplitude versus Time, LAX Theme Building Ambient Data 19 Oct. 2007 Channel=40



### Sample Data, Acceleration (g)







### Sample Data, Displacement (inch)

# Sample Ambient Vibration Spectra, Top of Core, X and Y Directions



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# **2007 Pre-Retrofit Results**

Frequency	Shape	Damping, Ambient	Damping, Shaker
2.4 Hz	N-S	1%	5%
2.6	E-W	2%	5%
4.7	Torsion + Legs		

# **2010 Post-Retrofit Results**

Frequency	Shape	Damping, Ambient	Damping, Shaker
1.8 Hz	N-S	1%	tbd
1.9	E-W	2%	tbd
4.7	Torsion + Legs		

# Tuned-Mass Damper 90-Kip Snapback Tests


## NEES Aftershock Monitoring of Reinforced Concrete Buildings in Santiago, Chile following the February 27, 2010 Mw=8.8 Earthquake



#### **Project Collaborators and Contributors:**

Aziz Akhtary (Grad Student Researcher, CSU Fullerton) Juan Carlos de la Llerra (Dean, Catholic University of Chile, Santiago) Anne Lemnitzer (Assist. Prof, Cal State Fullerton) Leonardo Massone (Assist. Prof., Univ. of Chile, Santiago) Bob Nigbor (NEES@UCLA co-PI & Manager) Derek Skolnik (Sr. Project Engineer, Kinemetrics) John Wallace (Professor, UCLA and NEES@UCLA PI)

#### Preparation of Instrumentation Layouts Equipment provided by NEES@UCLA





#### Instrumentation used:







## Instrumented Buildings Located in Santiago, Chile

#### Buildings selected based on:

-Access and permission -EERI Recon Team input -Typical design layouts representative for Chile and the US -Local collaborator for building selection: Juan Carlos de la Llerra



Ambient Vibration 2 Aftershocks Ambient Vibration 30 Aftershocks

Ambient Vibration 4 Aftershocks

## **Chile RAPID Instrumentation Team**

<u>US Team Members:</u> Anne Lemnitzer (CSUFullerton) Alberto Salamanca (NEES @ UCLA) Aditya Jain (Digitexx) Marc Sereci (Digitexx; EERI team member) John Wallace (UCLA, Instrumentation PI)

#### Local Graduate Student Members :

Matias Chacom, (Pontificia Universidad Católica de Chile) Javier Encina, (Pontificia Universidad Católica de Chile) Joao Maques, (Pontificia Universidad Católica de Chile)

#### Local Faculty Collaborators

Juan C. De La Llera M. (Pontificia Universidad Católica de Chile) Leonardo Massone (University of Chile, Santiago)

<u>CO-Pls on the NSF Rapid Proposal</u> Robert Nigbor (UCLA) John Wallace (UCLA)







#### <u>Building B:</u>

- -10 story RC residential building
- Structural system: Shear Walls
- -Post Earthquake damage:
- I. Shear wall failure,
- II. Column buckling,
- III. Extensive non-structural failure,
- IV. slab bending & concrete spalling

<u>Observed Damage in the 10 story shear wall</u> <u>building:</u>

Repetitive Damage at the -1 level (Parking level): Wall-Slab intersections





## 1<sup>st</sup> floor shear wall damage



# 1<sup>st</sup> floor shear wall damage



# Column buckling at first floor



# Shear Wall Instrumentation with LVDTs



#### Story Accelerations 2010 05/02 14:52:39 UTC M5







### Shear and Flexure Deformations





Figure 4: Shear-flexure interaction for a wall subject to lateral loading. (adapted from Massone and Wallace, 2004)

### LVDT Measurements



#### Shear and flexure deformations



The rotation for flexure was taken at the base of the wall (so the top displacement is multiplied by the wall height), which is the largest value expected for flexure. If we assume that the flexure corresponds to a rotation at wall mid-height, the flexural component should be multiplied by 0.5.



Rocking about the x axis = orientation of shear wall (corresponds to shear wall cracking)

## Particle Motion



#### Lessons Learned in Chile & Turkey Deployments

- Airport regulations (invitation letters, label equipment as non stationary)
- Trigger and record mechanisms (Continuous for short duration, triggered for longer)
- Instrumentation cabling (<100m, Power supplies)</li>
- Time Frame (ambient + aftershocks, can be 1-day or months)
- Battery power is workable
- Local collaboration essential (building access, installation, translations)
- Equipment Transportation (baggage is simple if possible)

#### Experimental Modal Analysis and Aftershock Monitoring for Two Christchurch Structures

Jose Restrepo, UCSD David Deutsch, USC Bob Nigbor, NEES@UCLA Matt Schoettler, UCSD Sahin Tasligedik, Univ. of Canterbury

August 4, 2011







## **Two Structures**

- St. George Hospital Carpark
  - 5-level parking garage
  - Modern reinforced concrete construction
  - Damaged shear walls
  - Being repaired
  - Not in use until repairs completed
- o Ibis Hotel
  - Inside Central Business District "Red Zone"
  - 9-story hotel building
  - Modern reinforced concrete construction
  - Extensive damage
  - Will be repaired















## SGCP Measurements

- o 12-channel system only
- 7 configurations for ambient
  vibrations, every corner of structure
  XYZ with CP5 level reference
- 20 minutes of ambient data per configuration
- Earthquake monitoring over 2 nights,
  7 aftershocks recorded

## Sample ambient accelerations CP5 Column G2, Y



## Sample PSD, X-direction, CP5 Column G2



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# Sample PSD, Y-direction CP5, column G2



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# Earthquake Data (130 sec, 11.8 cm/s/s)



#### **Preliminary Ambient Results, SGCP**

Frequency	Period	Shape	Damping
Hz	Sec	Qualitative	Preliminary
2.9	.34	N-S 1	3%
3.2	.31	E-W 1	2%
4.5	.22	Torsion	
9.1	.11	N-S 2	
10	.1	E-W 2	


## Next Up in August– Christchurch Women's Hospital



## Thanks!

## • For more information:

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